ENGINEERING QUICK REFERENCE GUIDE_

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BRIDGING





DISCLOSURE

The construction principles discussed within the pages of this manual are to be viewed as a reference source only. All materials contained within this manual, including but not limited to, calculations, details and schedules, are not intended to set limitations on design nor be interpreted as being the only design criterion. Structural design should be in accordance with local Building Code considerations and therefore, may not necessarily follow the guidelines suggested within this manual.

ACCREDITATION

All engineering calculations, equations, examples and details cited in this manual have been compiled by: Namdar Structural Engineering, Inc., Riverside, California. 2002.



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Introduction

SECTION 1

INTRODUCTION

The following sections contain applicable load information from the 1994 Uniform Building Code, design procedures, examples and tables for Allowable Uniform Loads, Concentrated Loads, Shear Walls, Lintels, Retaining, and Basement Walls.

The THERMOWALL PS® building system can be used for: exterior load bearing walls under axial and lateral loads. Interior load or non-load bearing walls, shear walls to resist in-plane shear loads, slender walls with combined eccentric axial and out-of-plane loads, columns carrying concentrated loads, especially on the side of openings partition walls curved walls lintels and beams carrying gravity and lateral loads elevated slabs with reinforcement post tensioned slabs carrying gravity and lateral loads basement and retaining walls foundation stem walls free-standing fences and planters, curved or straight roofs, flat or gabled.

Note:

Based upon full-scale testing the following factors have been generated:

Shear Walls: Strength Reduction Factor ϕ := .85 Perform Wall Shape Reduction Factor I := .85

Slender Walls: Equivalent Thickness 4.75 for 6" diameter core

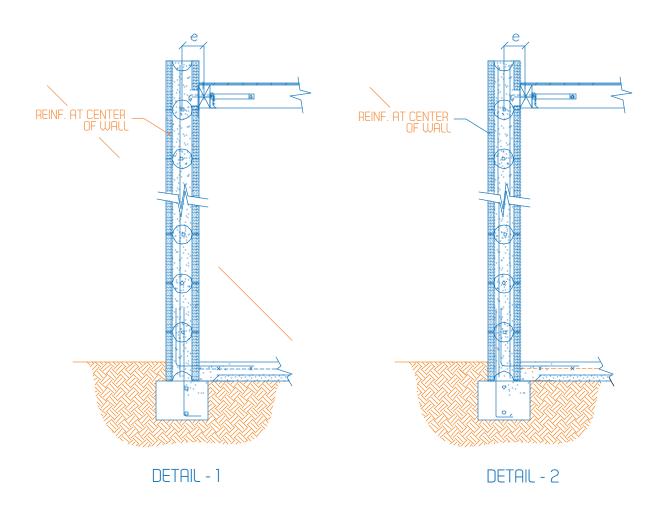
Shape Reduction Factor 6.75 for 8" diameter core

DESIGN TABLES

The following tables are an outline of possible designs and load bearing capacities. High exposures to wind and seismic loads have been assumed. The tables, however, are not supposed to set limitations to design possibilities. For various exposures, see equations in Chapter 3, "Design Procedures" of the comprehensive Engineering Design Manual.

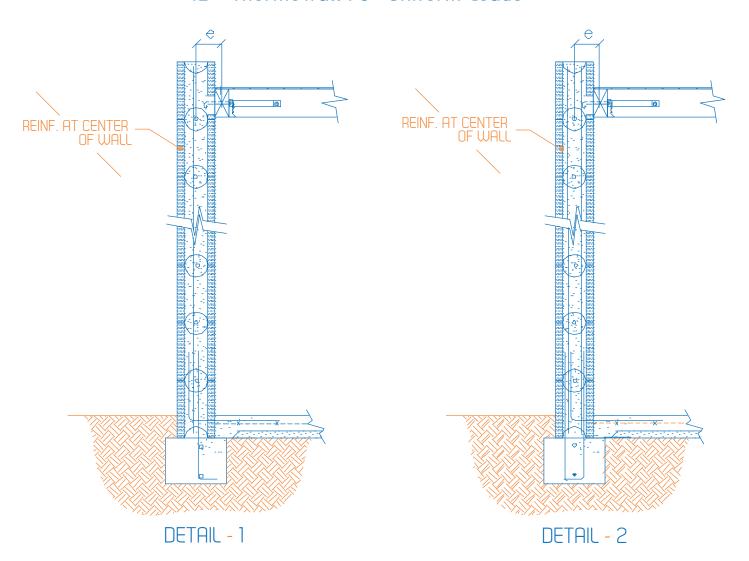
SECTION 2 - SCHEDULES

10" Thermowall PS® Uniform Loads



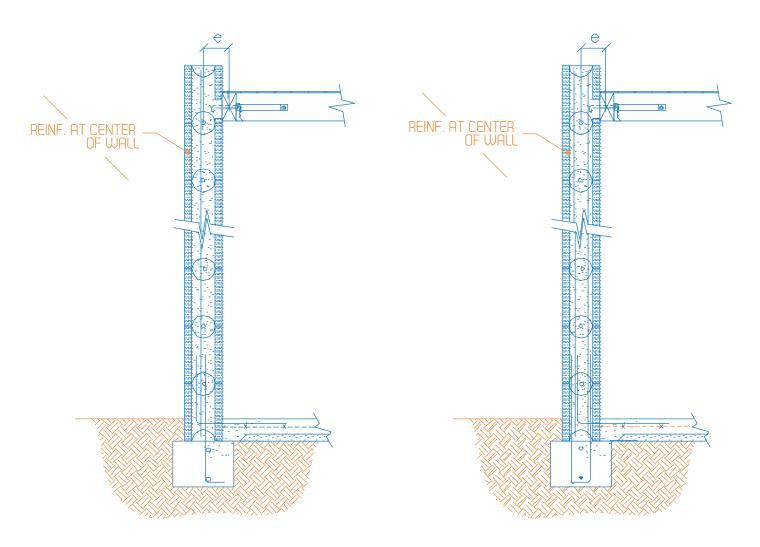
	ALLOWAE	DESIGN PARAMETERS				
HEIGHT OF	70 MPH (1		SPACING 80	MPH (28.86 PSF	1	e=6 3/4"
WALL FT.	#4 @ 150 0.	C #5 @ 15o O.C	150 O.C #5 @ 150 O.C #5 @ 150 O.C #6 @ 150 O.C (EACH FACE)			STEEL:
8'-0"	1,000	2,100	1,800	2,700	3,000	60,000 psi
10'- 0''	800	1,800	1,500	2,500	2,700	CONCRETE
12'- 0"	600	1,500	1,200	2,400	2,500	
14'- 0"	400	1,200	<i>7</i> 50	2,200	2,400	2,5 00 psi
16'- 0''		600	400	1,500	1,600	SEISMIC ZONE
18'- 0''		600 1,500		1,500	ZONE - 4 . % = 0.40	
20'- 0"			-			
22'- 0"			WIND EXPOSURE			
		SEE DETAIL - 1		SEE DE	TAIL - 2	EXPOSURE - C

SECTION 2 - SCHEDULES 12" Thermowall PS® Uniform Loads



	ALLOV	DESIGN				
	ALLOWABLE	AXIAL LOAD IN	POUNDS PER LINI	EAL FOOT		PARAMETERS
		Rebar 8	SPACING			ECCENTRICITY:
HEIGHT OF	70 MPH (16.	Ø3 PSF)	80	MPH (28.86 PSF)	e = 7 3/4"
WALL FT.	#4 @ 15o O.C	#5 @ 15o NC	#5 @ 15o O.C	#5 @ 15o O.C	#6 @ 15o O.C	6 – 1 – 1 – 1 – 1 – 1 – 1 – 1 – 1 – 1 –
	#7 @ 130 U.C	#0 @ 100 0.0	#0 @ 100 0.0	(EACH FACE)	(EACH FACE)	STEEL:
8′-0″	1,400	2,400	2,400	3,400	3,600	60,000 psi
10'- 0"	1,400	2,000	1,800	3,000	3,400	CONCRETE
12´- 0´´	800	1,700	1,400	3,000	3,200	
14'- 0''	400	1,200	800	3,000	2,5 00 psi	
16'- 0''	300	800	300	1,200	1,800	SEISMIC ZONE
18'- 0"		300 1,600		20NE - 4, % = 0.40		
20'- 0"			LUIND EVDOCUDE			
22'- 0"			WIND EXPOSURE			
		SEE DETAIL - 1		SEE DE	TAIL - 2	EXPOSURE - C

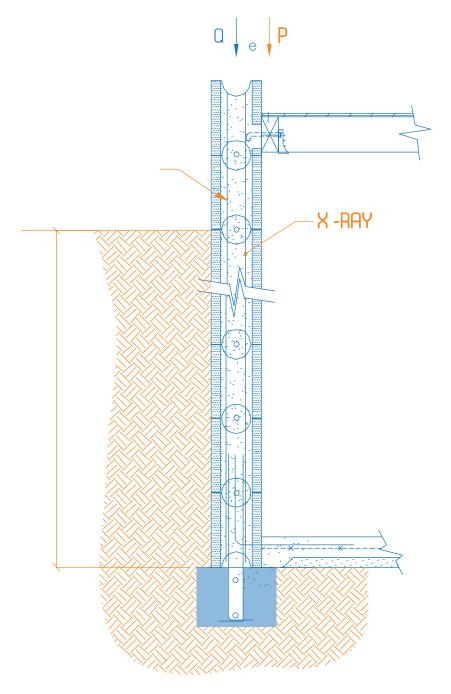
SECTION 2 - SCHEDULES 14" Thermowall PS® Uniform Loads



DETAIL - 1 DETAIL - 2

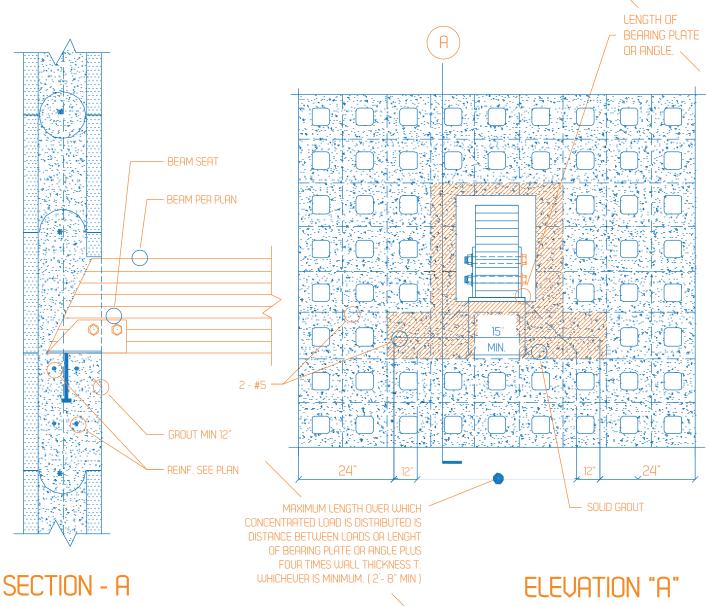
	ALLOV	DESIGN PARAMETERS				
	HLLUWHBU	= HXIHL LUHD IN	POUNDS PER LINI	EHL FUU I		PHRHMCTCRS
		REBAR 8	SPACING			ECCENTRICITY:
HEIGHT OF	70 MPH (16.	03 PSF)	80	MPH (28.86 PSF)	e = 8 3/4"
WALL FT.	#4 @ 15o O.C	#5 @ 15o O.C	#5 @ 15o O.C	#6 @ 150 O.C (EACH FACE)	STEEL:	
8'-0"	1,400	2,000	2,000	3,000	3,200	60,000 psi
10'- 0''	1,400	1,800	1,600	2,800	3,000	CONCRETE
12'- 0"	800	1,400	1,200	2,600	2,800	
14'- 0''	400	1,000	800	2,000	2,400	2,5 00 psi
16'- 0"		600	200	1,000	1,400	SEISMIC ZONE
18'- 0"		1,200				ZONE - 4 . % = 0.40
20'- 0"			1,000	LUINID ELIDOCUIDE		
22'- 0"		WIND EXPOSURE				
		SEE DETAIL - 1		SEE DE	TAIL - 2	EXPOSURE - C

SECTION 2 - SCHEDULES



	E	DESIGN PARAMETERS								
RETAINING	ECCENTRIC	AXIAL CENTRIC			OOLIODETE.	EQUIVALENT	ECCENTRICITY			
HEIGHT	AXIAL LOAD (P)-(PLF)	LOAD (Q)-(PLF)	X - BAR	X-BAR Y-BAR	Y - BAR	CONCRETE	FLUID PRESSURE	e = 73/4		
		(4)-(14)				Phessune	CONCRETE			
3'-0"	500		1-#4 @	1-#5 @	2,500 PSI		PER SCHEDULE			
4'-0''	600	> 10,000	15" O.C	15′ O.C	2,000101	50 PCF	PCR SCHEDUCE			
6'-0"	700	≥ 10,000	1-#6 @	3 @ 1-#6 @	#6 @ 1-#6 @	1-#6 @ 1-#6 @	1-#6 @ 1-#6 @	, , , , , , , , , , , , , , , , , , ,	JU PLF	STEEL:
8'-0"	600		15" O.C 7.5" O.C 4,0		4,000 PSI		60,000 psi			

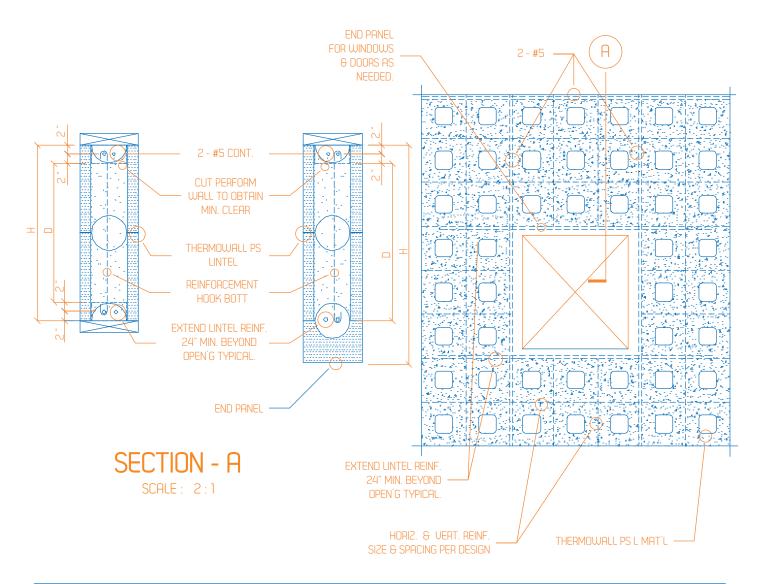
SECTION 2 – SCHEDULES ALLOWABLE CONCENTRATED LOADS FOR 10." 12 " AND 14" PANELS



SCALE :	2:1	

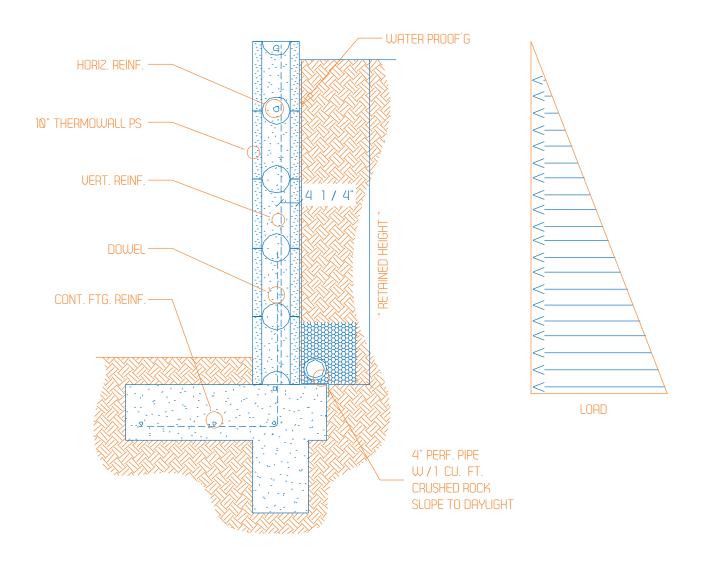
1	ALLOWABLE OWABLE AXIAL		DESIGN PA	ARAMETERS		
HEIGHT OF	#4 @	#5 @	ECCENTRICIT	Y: e= 4"		
WALL FT.	15o O.C	15o O.C	ea. Face	ea. Face		
8'-0"	3800	6000	7400	8000	CONCRETE	STEEL:
10′-0′′	3800	4800	6400	7400	2,5 00 psi	60,000 psi
12'-0"	2200	3800	6000	7000		
14′-0″	1200	2800	5000	6500	SEISMIC ZONE	WIND EXPOSURE
16′-0″		1600	2600	3700		
18'-0"				3200	ZONE - 4, % = 0.40	EXPOSURE - C
20'-0"				2700		

SECTION 2 - SCHEDULES LINTEL SCHEDULE



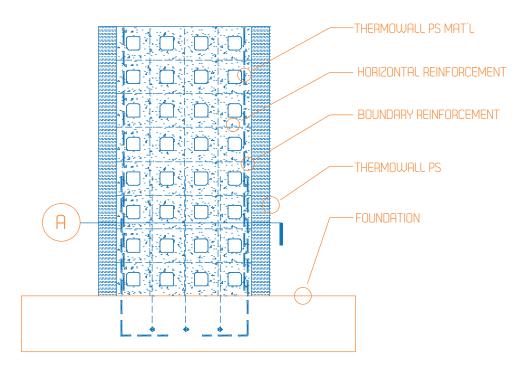
	FOR	DESIGN PARAMETERS					
	UNIFORM L	OAD (PLF)	TOP. REINF.	BOTT. REINF.	REMARKS	CONCRETE	
MAX. SPAN	15" LINTELS H=15" . D=13"	30" LINTELS H=30" . D=28"				2,5 00 psi	
3'- 0"	1600	2600	60.0 2- #5 2- #5 24"BEARING			STEEL:	
4'- 0"	1400	2000				60,000 psi	
6'-0"	800	1600		2- #5		SEISMIC ZONE	
8'- 0 "	600	1400			EHUH	EACH END	ZONE - 4 , % = 0.40
10'- 0''	500	1000				-	
12'- 0"	400	800				WIND EXPOSURE	
16'- 0"	200	600	•			EXPOSURE - C	
		UNL	ESS OTHERWIS	SE DRAW OR NOT	ED		

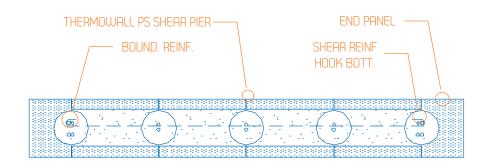
SECTION 2 - SCHEDULES RETAINING WALL SCHEDULE



RETAINING WALL SCHEDULE STEEL 60,000 PSI								
RETAINED HT.	SIZE	HORIZ. REINF.	VERT. REINF.	REMARKS	EQUIV. FLUID PRES			
3′ - 0″		1-#4 #5 @ 15o O.C						
4' - 0"	10″		#5 @ 150 0.C 2,500 psi CONCRETE 30 PCF (38 PCF FOR WALL)					
6' - 0"		1 - #5	#6 @ 15o O.C		(38 PUT FUR WHLL)			
8' - 0"			2-#6 15o 0.C	3,000 psi CONCRETE				

SECTION 2 - SCHEDULES SHEAR WALL SCHEDULE



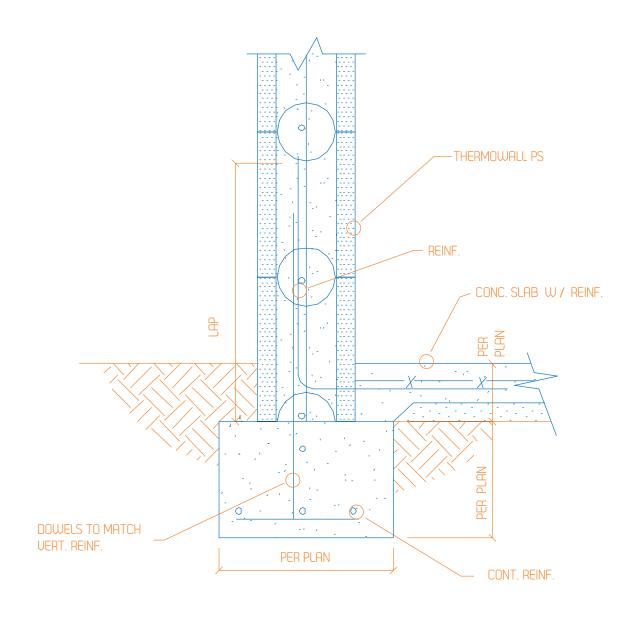


SECTION - A

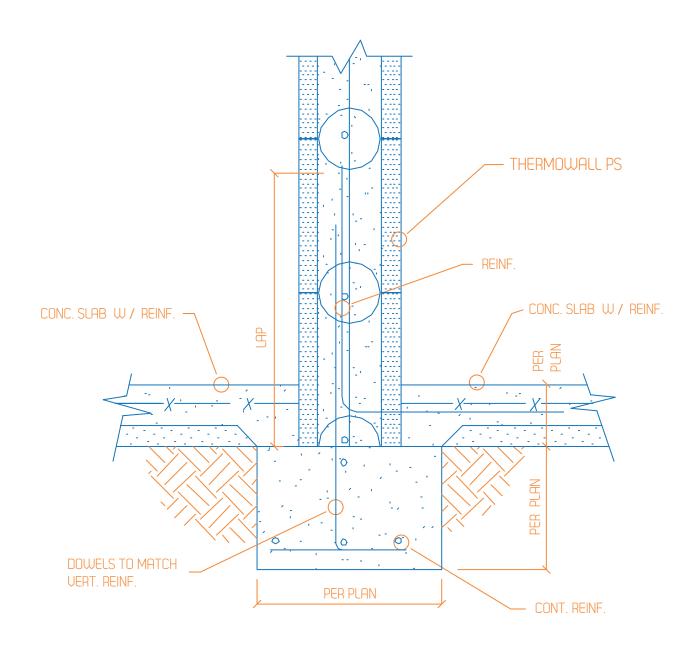
SCALE: 2:1

SHEAR WALL SCHEDULE FOR 8" OR 10"THERMOWALL PS - RESPECTIVELY				DESIGN PARAMETERS	
MAX. SPAN	LOADING (PLF)	BOUNDARY REINF.	REMARKS	CONCRETE	STEEL:
15"	2340	2 - # 5	#5 @ 15o O.C HORIZONTALLY		
30″	4690			2, 500 psi	80,000 psi
45"	7030			SEISMIC	WIND EXPOSURE EXPOSURE - C
60″	11 710				
<i>7</i> 5″	14 060				
120"	18 740				

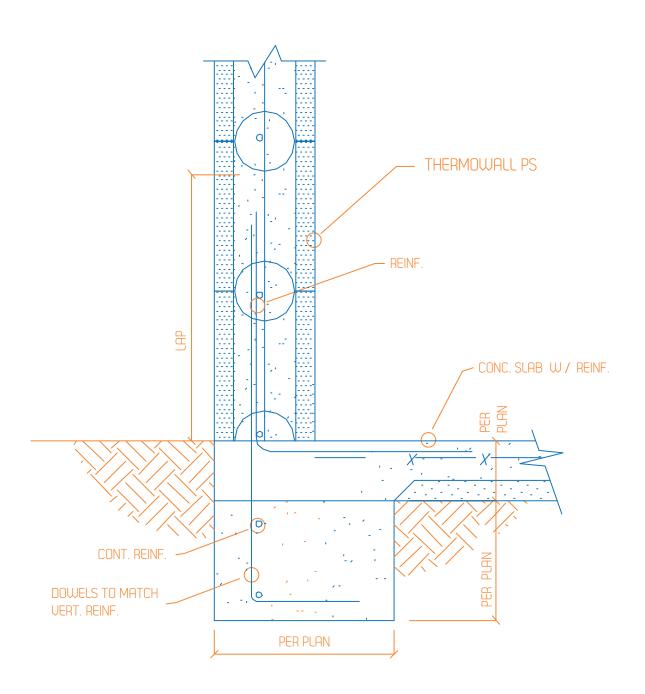
SECTION 3 - TYPICAL DETAILS 3.1 Foundation Details EXTERIOR FOOTING



SECTION 3 - TYPICAL DETAILS 3.1 Foundation Details INTERIOR FOOTING

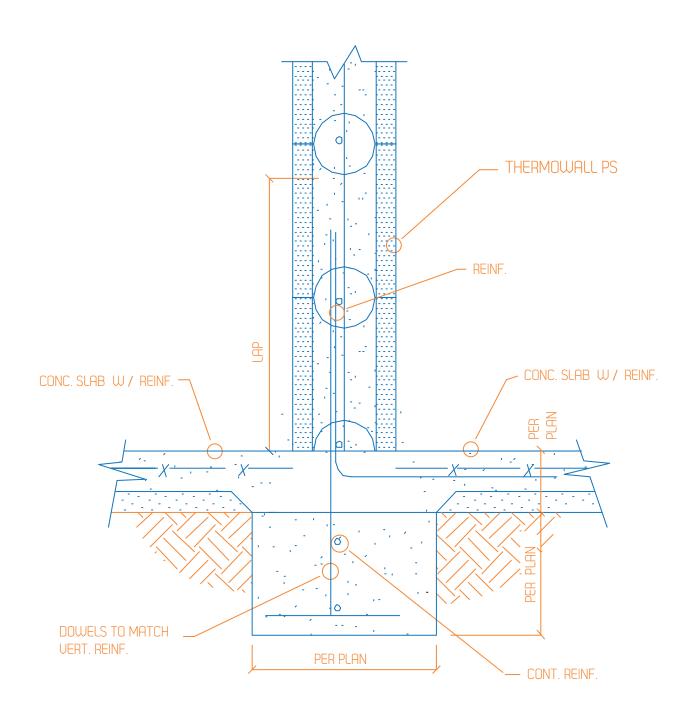


SECTION 3 - TYPICAL DETAILS 3.1 Foundation Details EDGE FOOTING

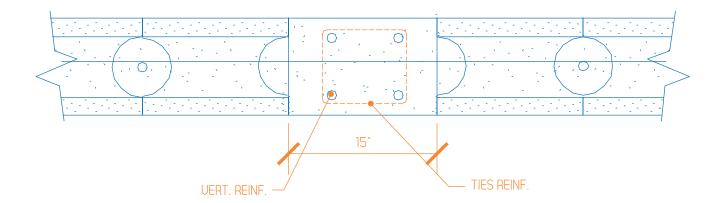


SECTION 3 - TYPICAL DETAILS 3.1 Foundation Details

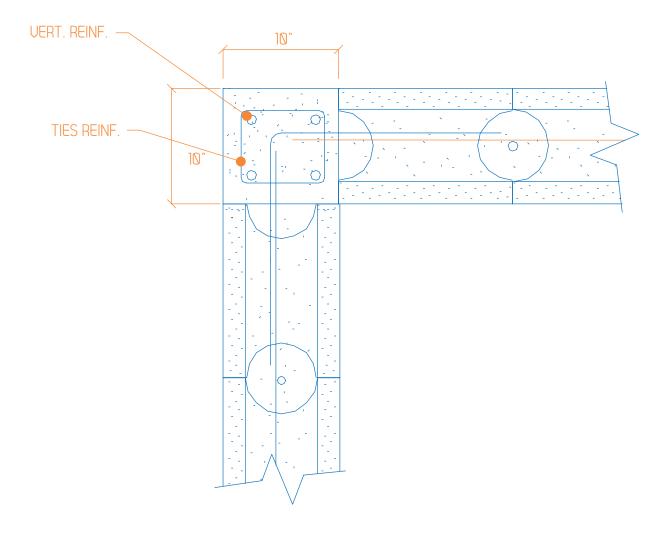
INTERIOR FOOTING WITH WALL RESTING ON SLAB



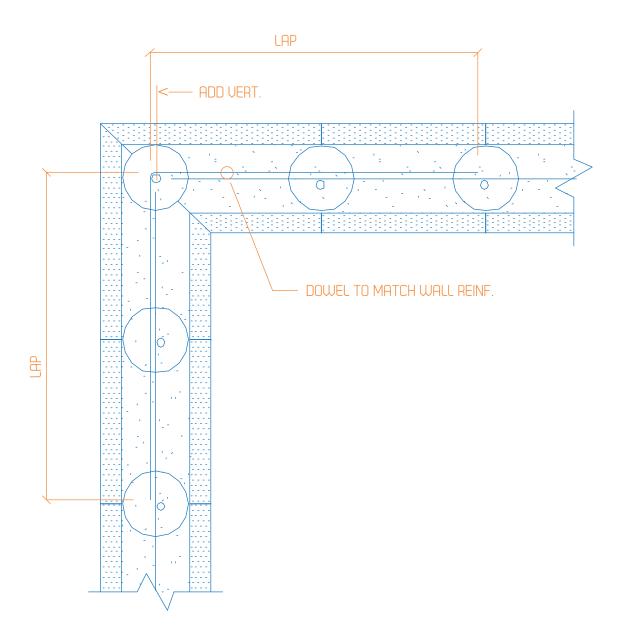
3.2 Pilaster Details FLUSH PILASTER



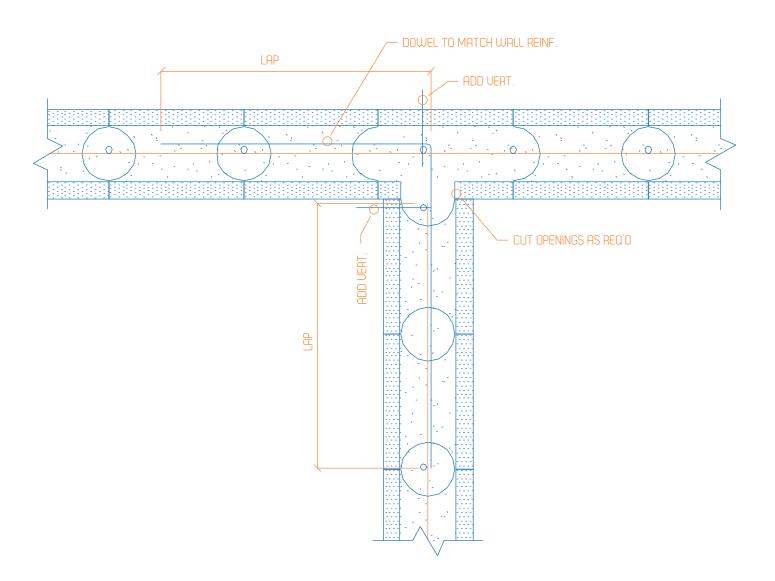
3.2 Pilaster Details FLUSH PILASTER AT CORNER



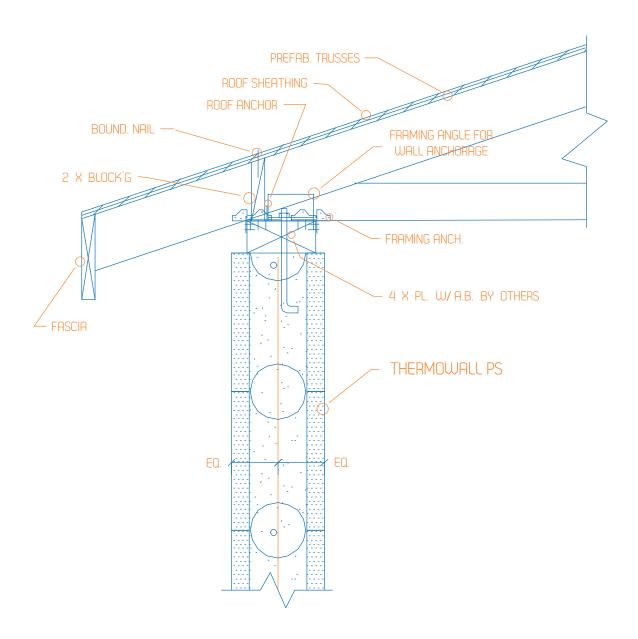
3.3 Wall to Wall Connection CORNER CONNECTION



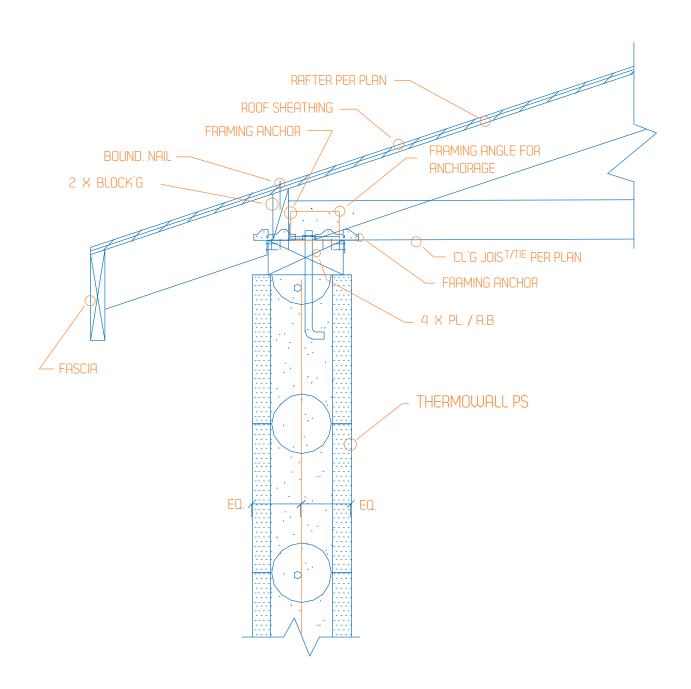
3.3 Wall to Wall Connection INTERIOR WALL TO EXTERIOR WALL CONNECTION



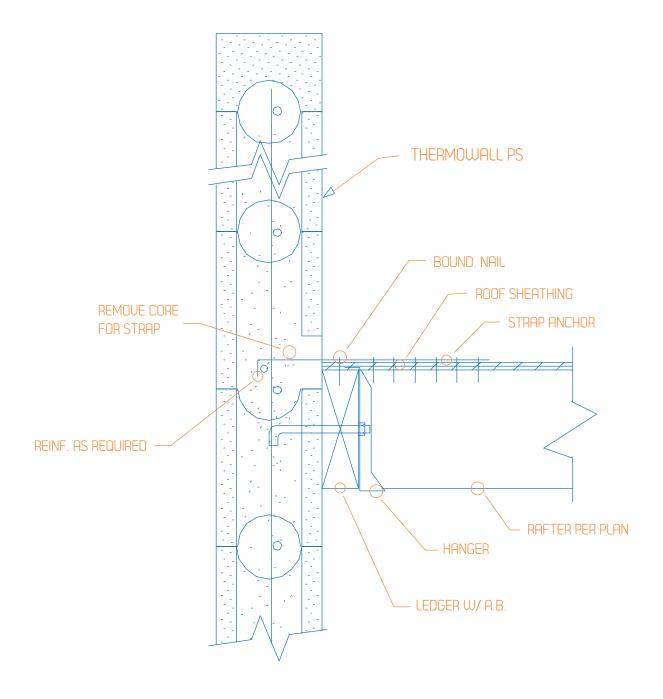
3.4 Roof to Wall Connection Details ROOF TRUSS / WALL CONNECTION



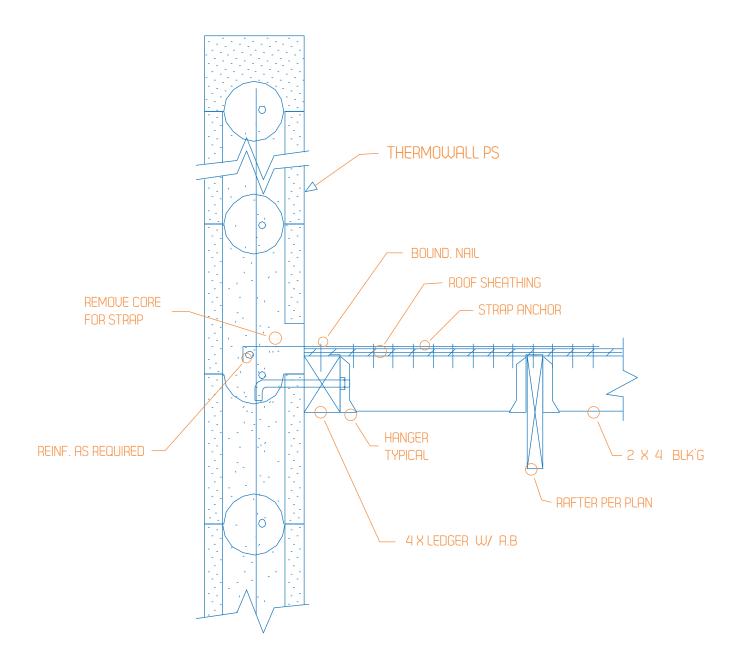
3.4 Roof to Wall Connection Details ROOF RAFTER / WALL CONNECTION



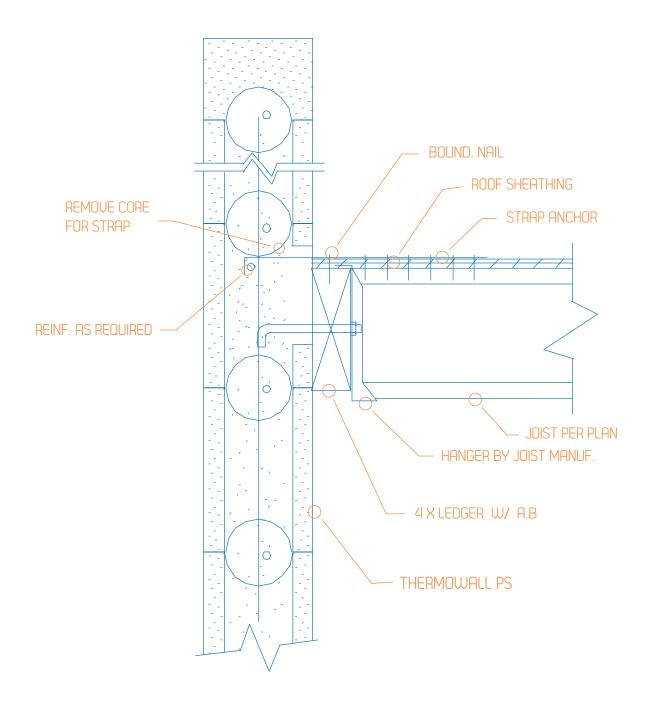
3.4 Roof to Wall Connection Details WALL WITH PARAPET / ROOF CONNECTION



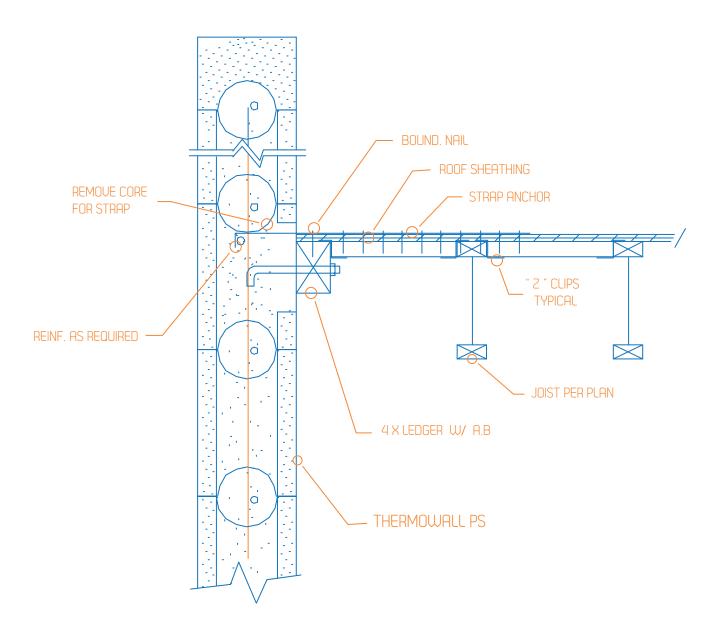
3.4 Roof to Wall Connection Details WALL WITH PARAPET TO ROOF CONNECTION, WALL PARALLEL TO JOIST



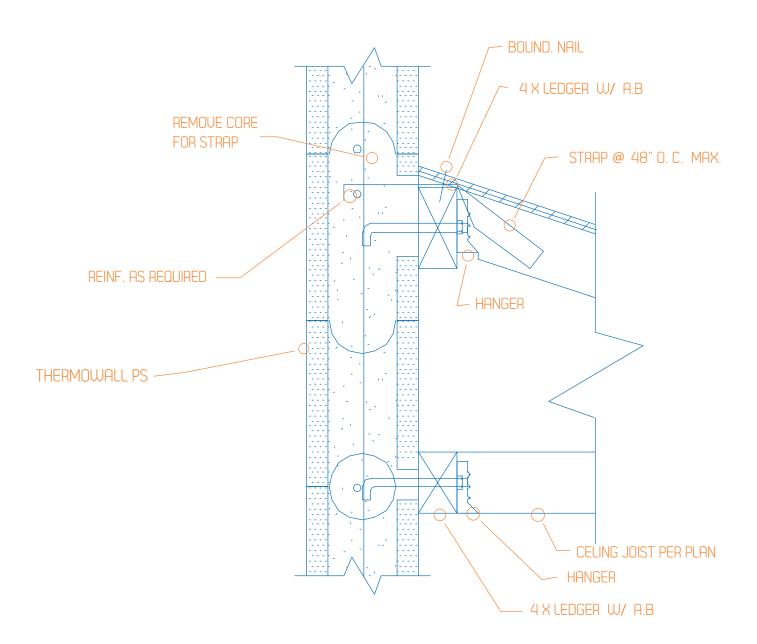
3.4 Roof to Wall Connection Details WALL WITH PARAPET / ROOF JOIST CONNECTION, JOIST PARALLEL TO WALL



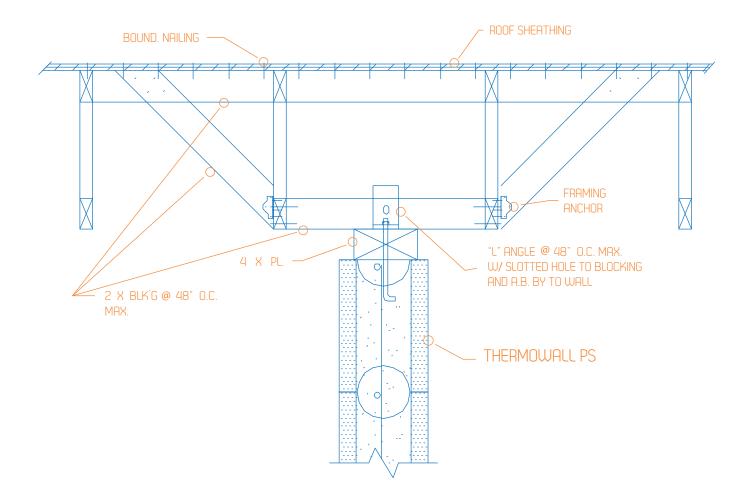
3.4 Roof to Wall Connection Details WALL WITH PARAPET / ROOF JOIST CONNECTION, PRE-FAB JOIST PARALLEL TO WALL



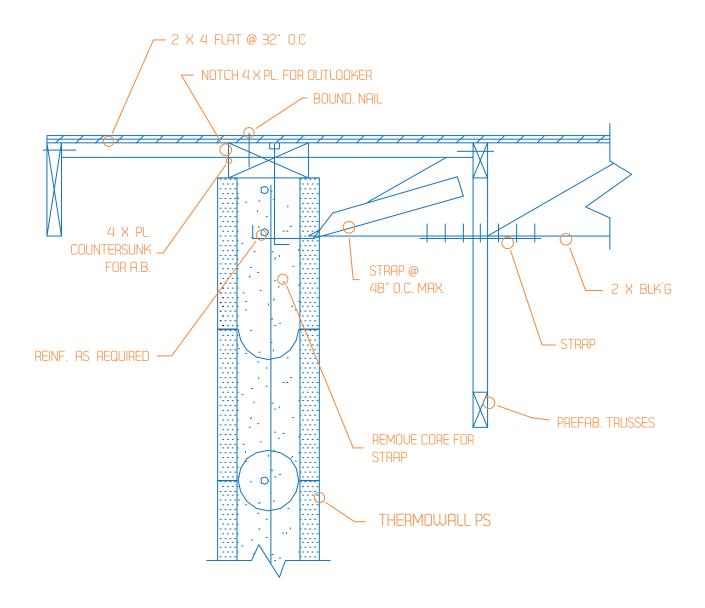
3.4 Roof to Wall Connection Details ROOF TO WALL CONNECTION

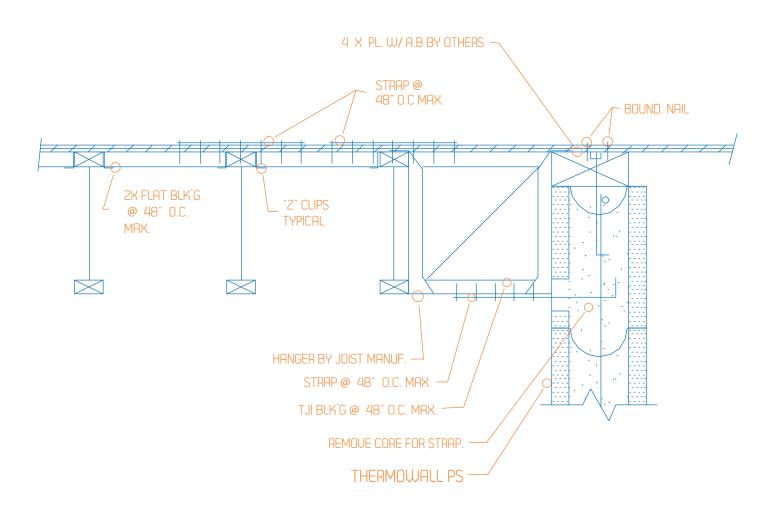


3.4 Roof to Wall Connection Details INTERIOR NON - BEARING WALL TO ROOF CONNECTION

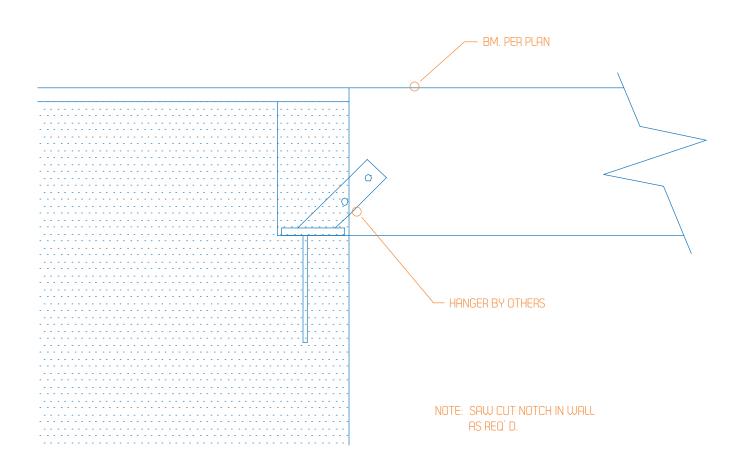


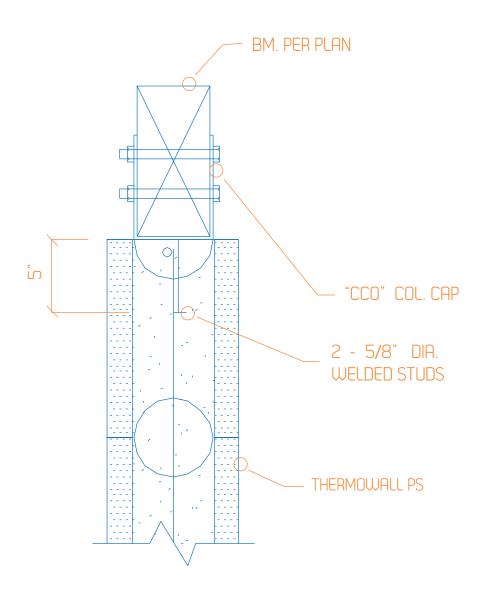
3.4 Roof to Wall Connection Details GABLE END WALL TO ROOF CONNECTION

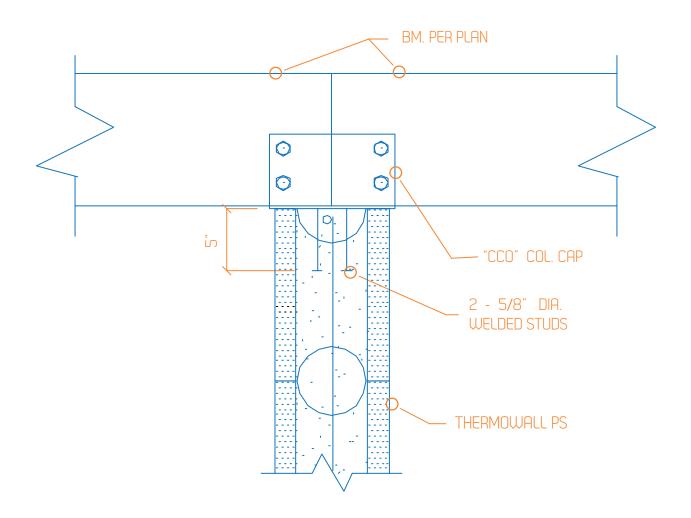




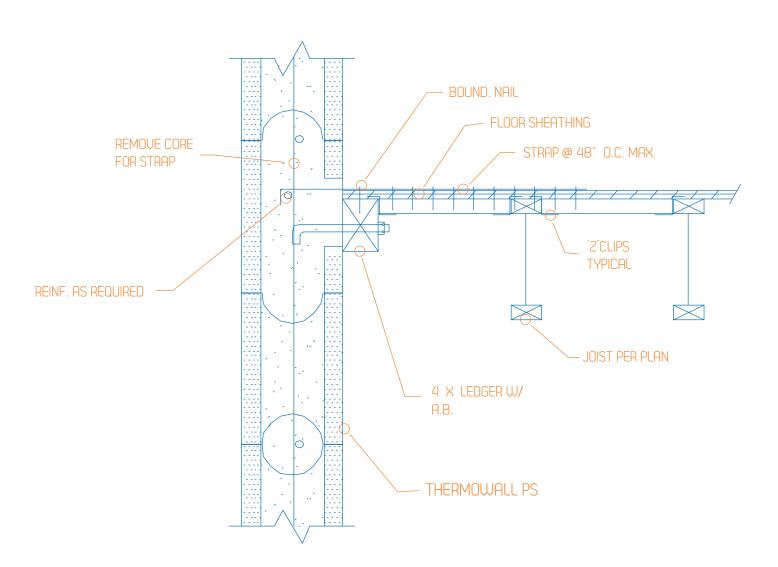
3.5 Wall to Wall Beam Connection BEAM / WALL CONNECTION



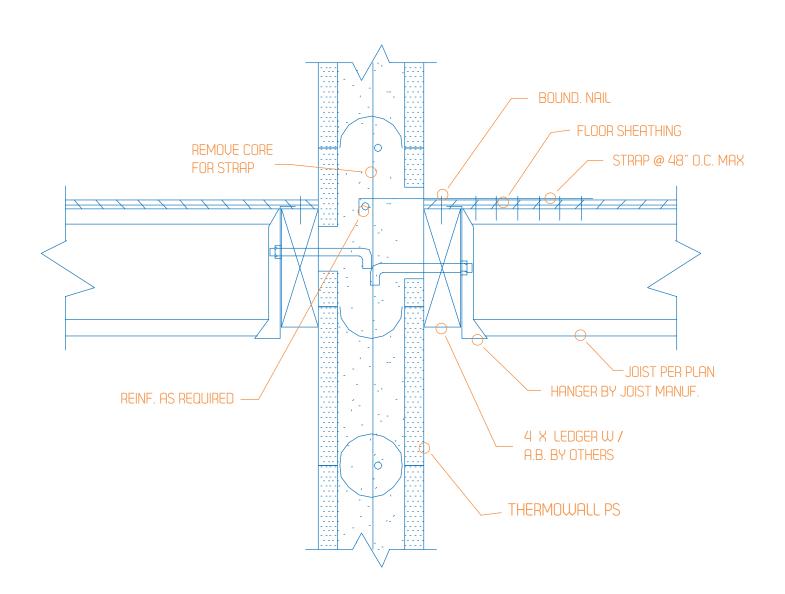




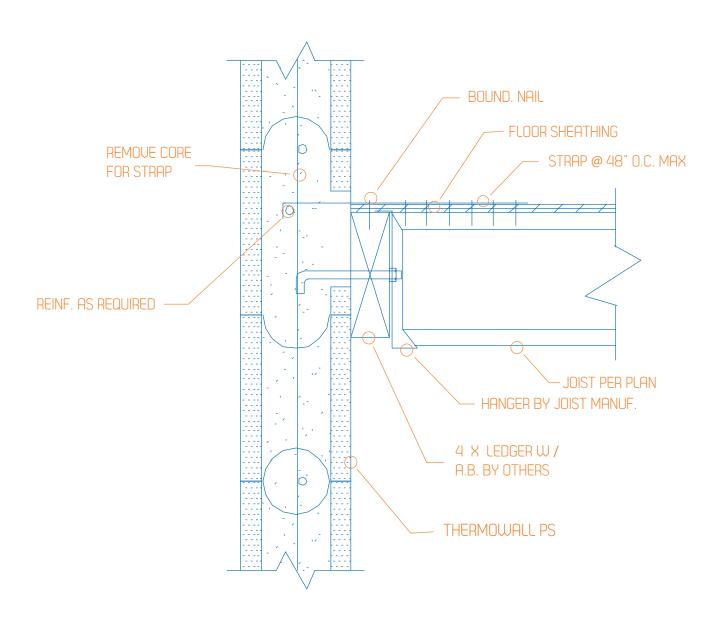
3.6 Floor to Wall Connection WALL TO FLOOR CONNECTION, WALL PARALLEL TO JOIST



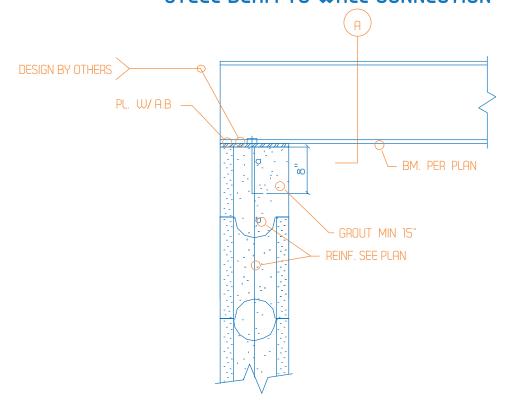
3.6 Floor to Wall Connection INTERIOR WALL / JOIST CONNECTION, WALL PERPENDICULAR TO JOIST



3.6 Floor to Wall Connection WALL TO FLOOR JOIST CONNECTION

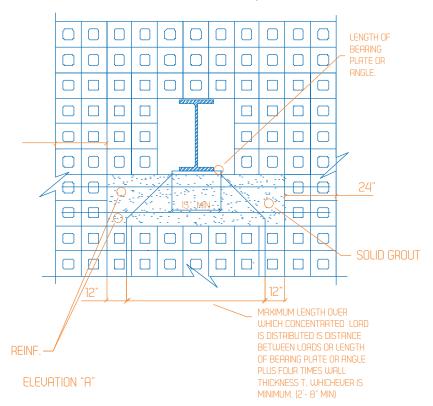


3.7 Steel Beam/Joist to Wall Connection STEEL BEAM TO WALL CONNECTION

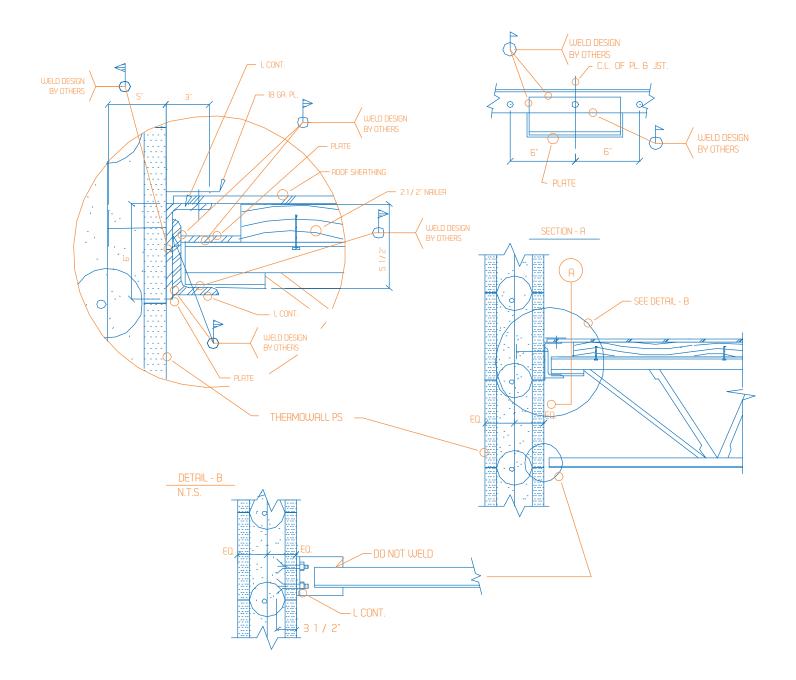


SECTION

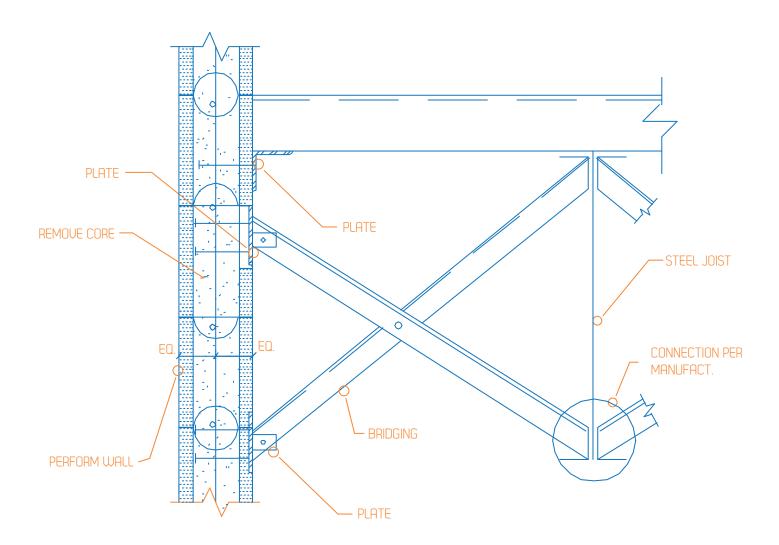
F't OF CONCRETE: 2,500 PSI F'c OF REINFORCING STEEL: 60,000 PSI



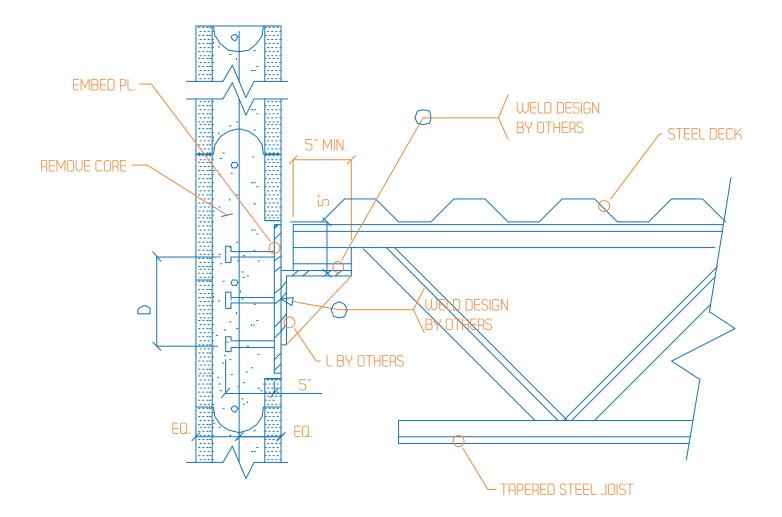
3.7 Steel Beam/Joist to Wall Connection STEEL TRUSS TO WALL CONNECTION



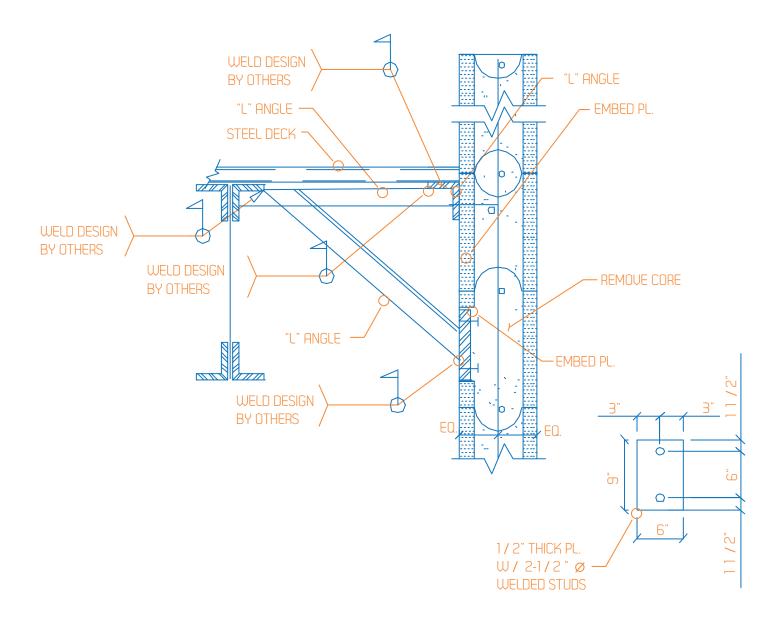
3.7 Steel Beam/Joist to Wall Connection STEEL BRACING / BRIDGING TO WALL



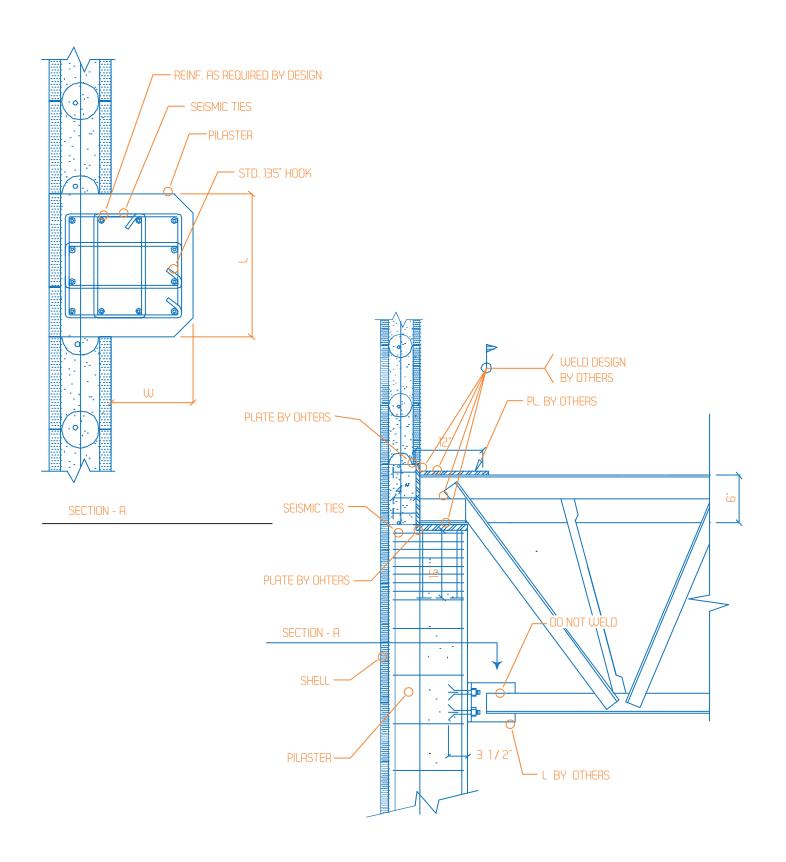
3.7 Steel Beam/Joist to Wall Connection STEEL JOIST TO WALL



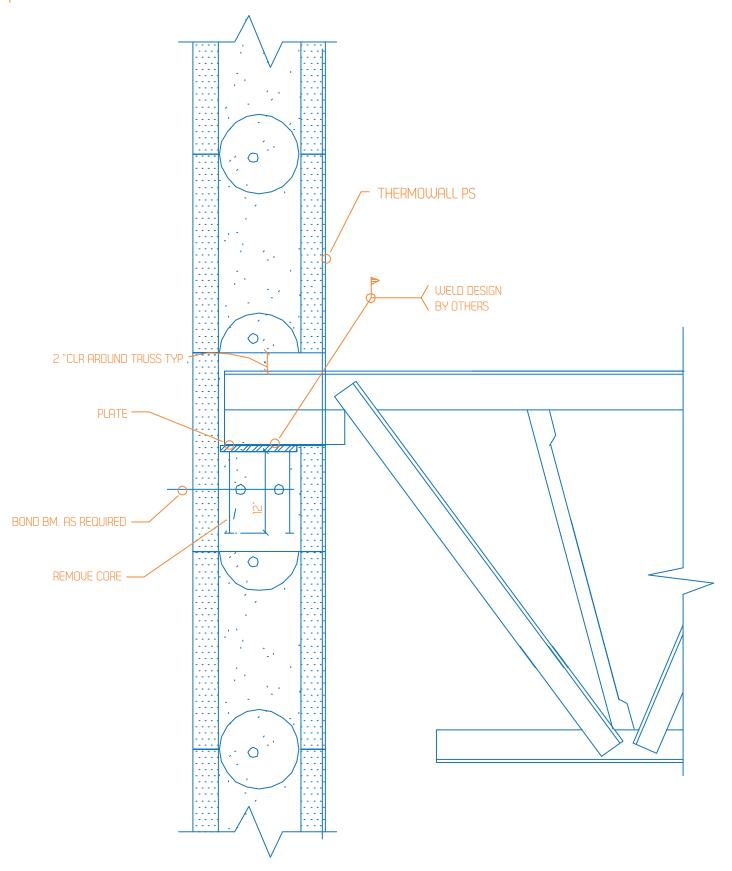
3.7 Steel Beam/Joist to Wall Connection STEEL BRIDGING TO WALL



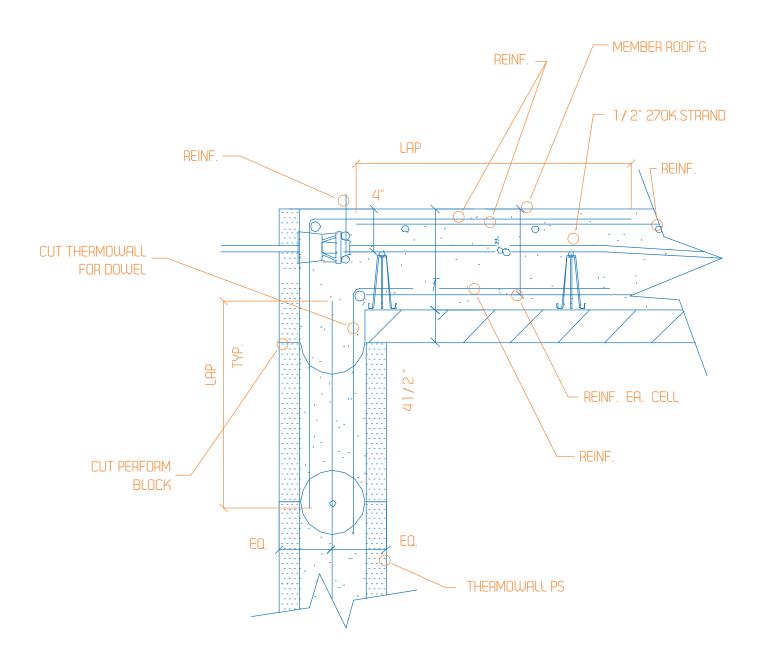
STEEL GIRDERS TO PILSTER



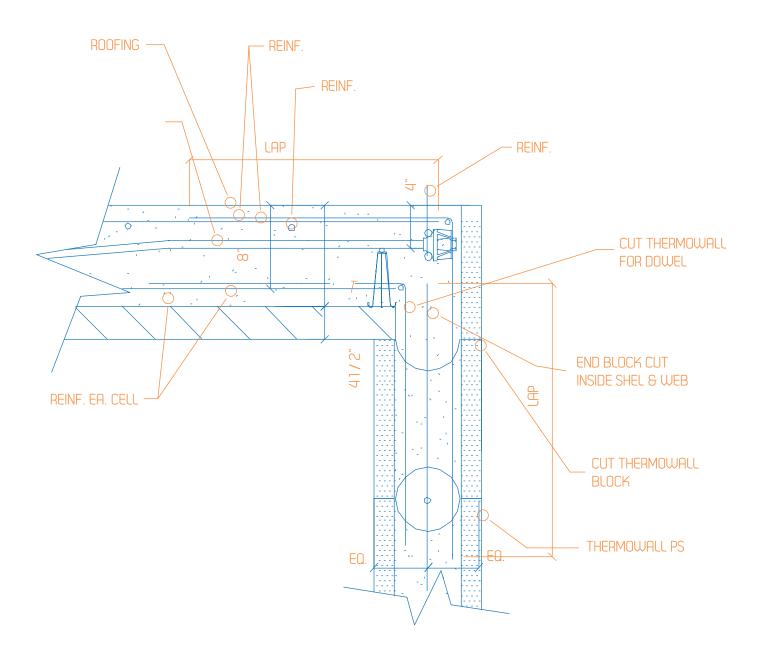
3.7 Steel Beam/Joist to Wall Connection TYPICAL STEEL JOIST TO WALL CONNECTION

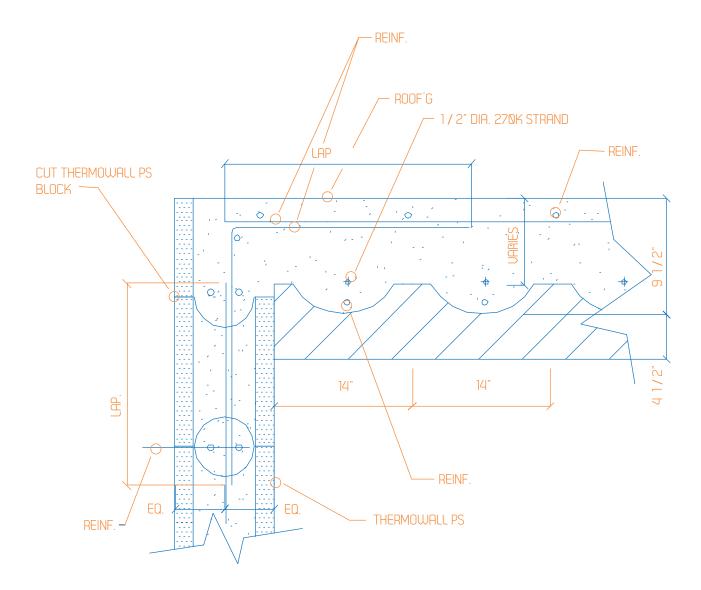


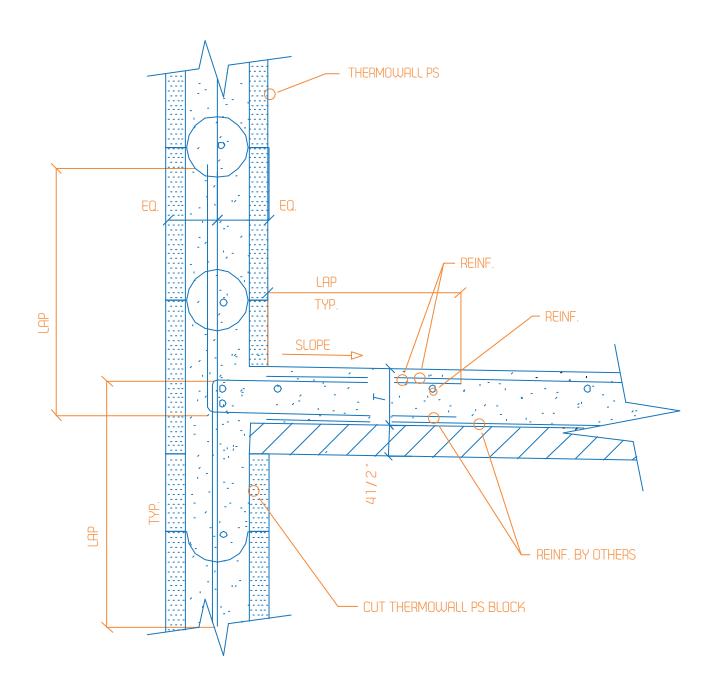
3.8 Concrete to Wall Connection POST TENSION SLAB TO WALL CONNECTION

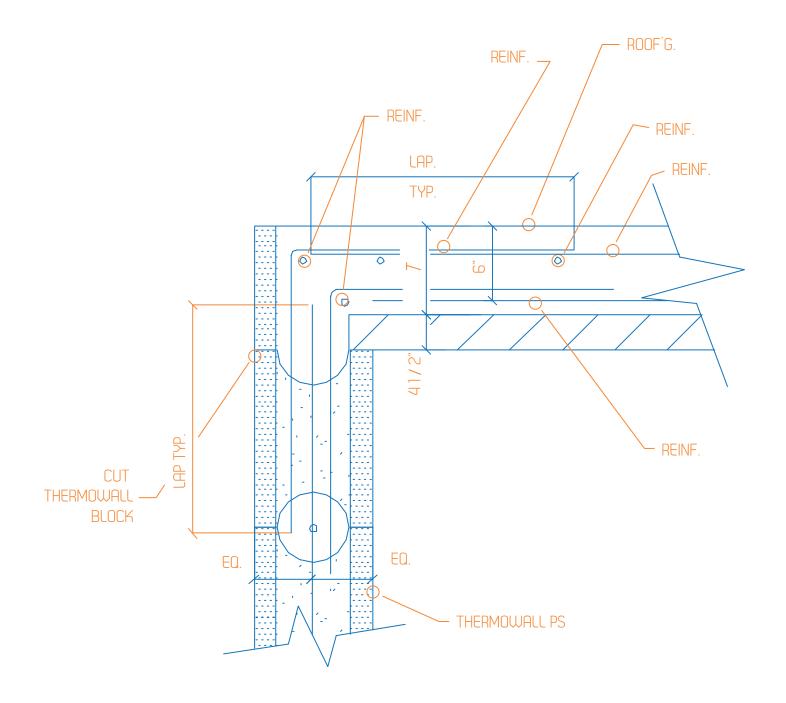


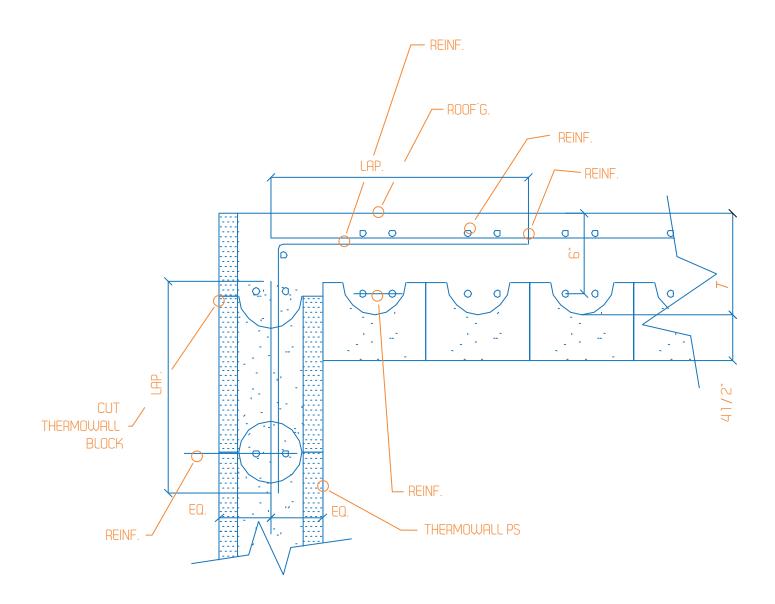
3.8 Concrete to Wall Connection POST TENSION SLAB TO WALL CONNECTION

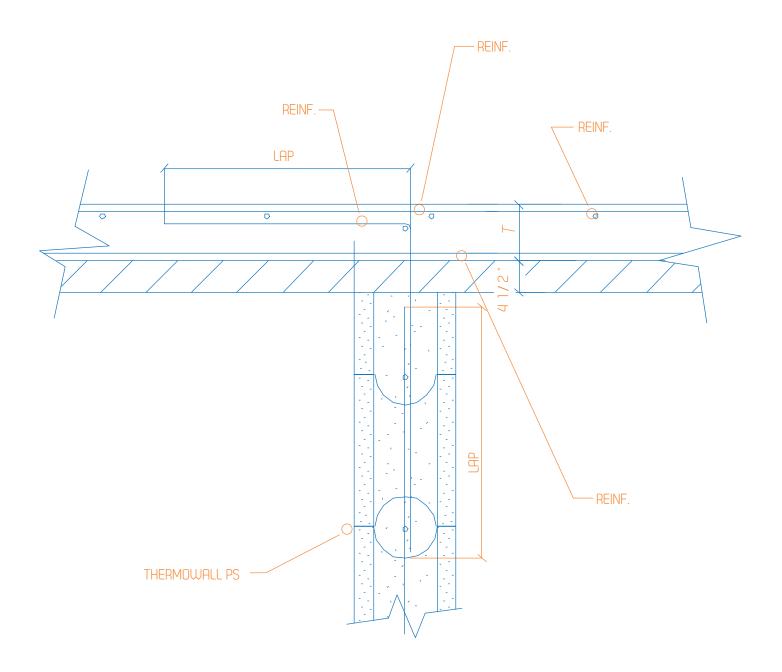


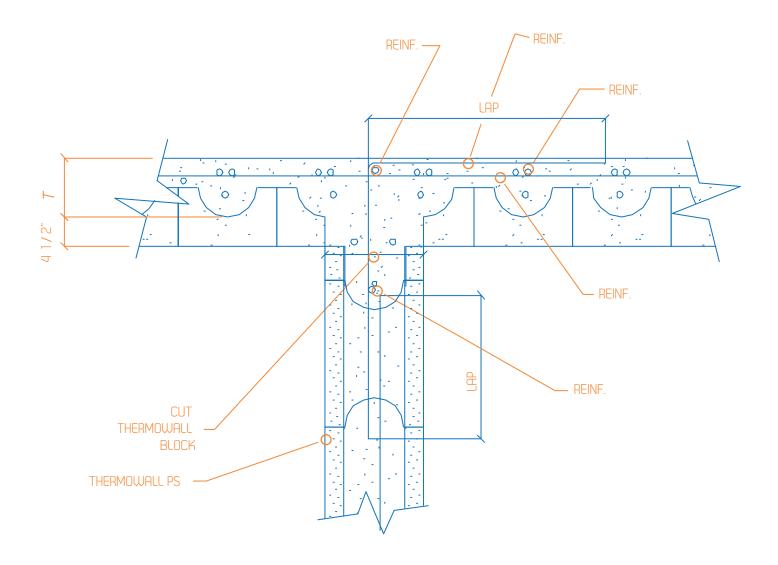


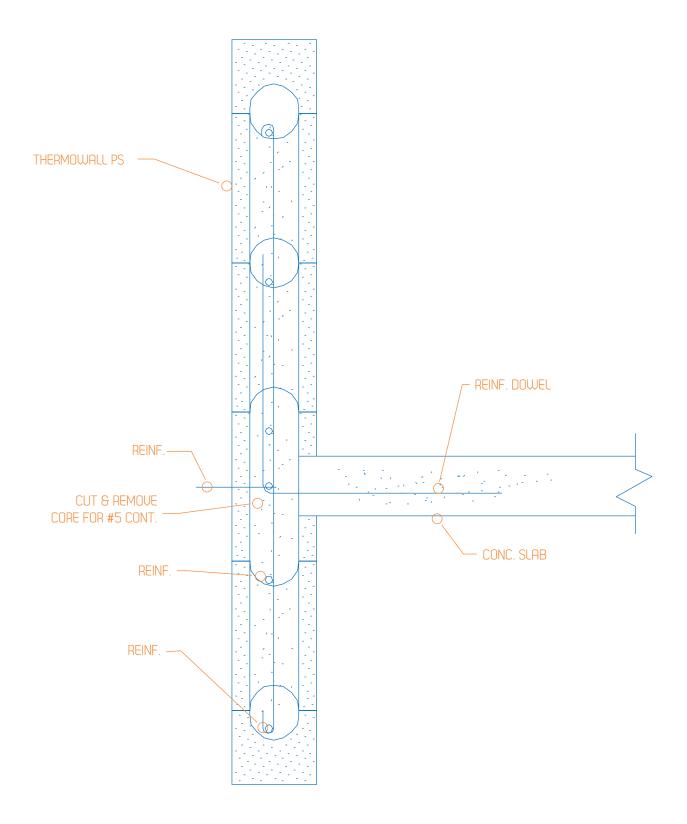


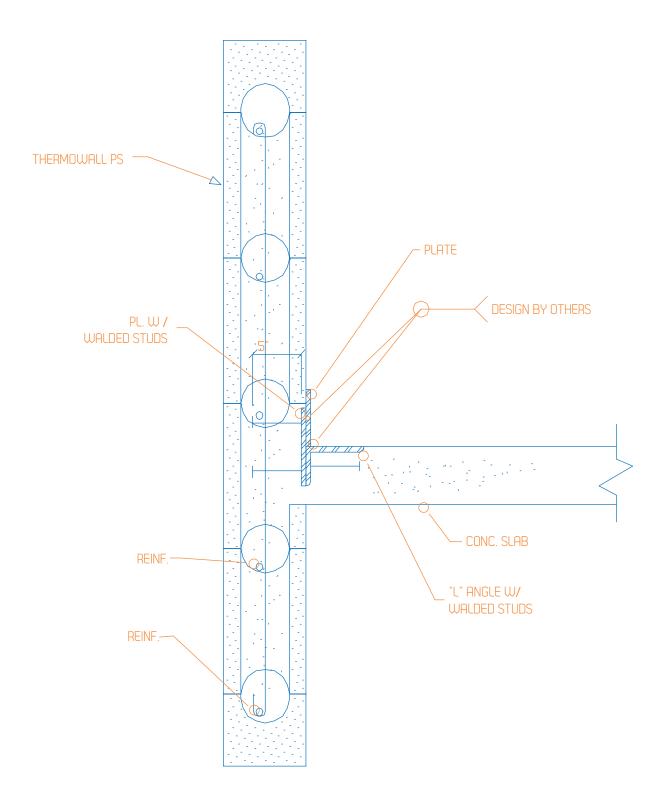


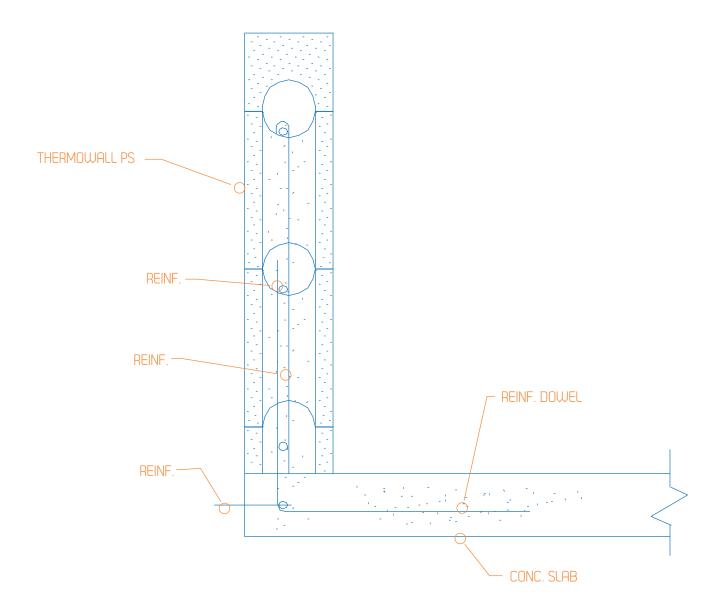


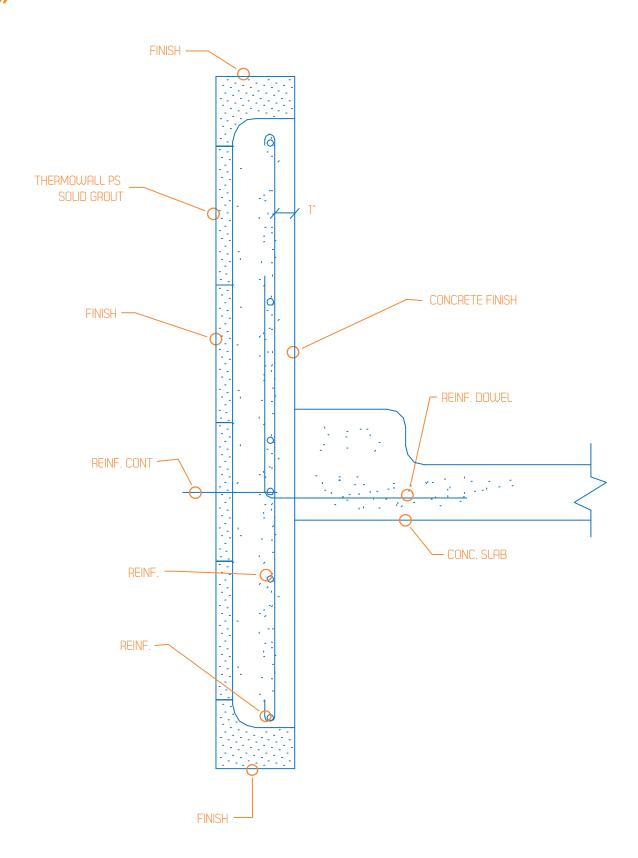


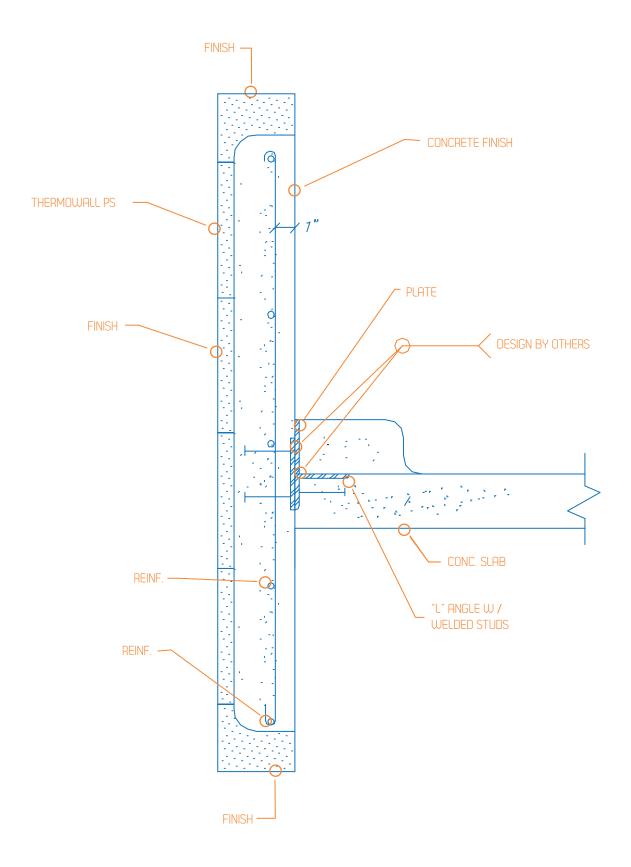


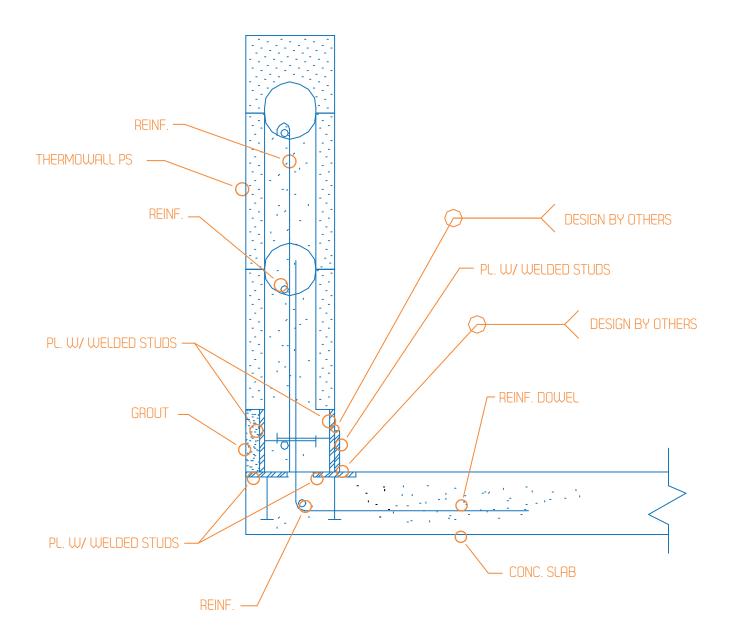




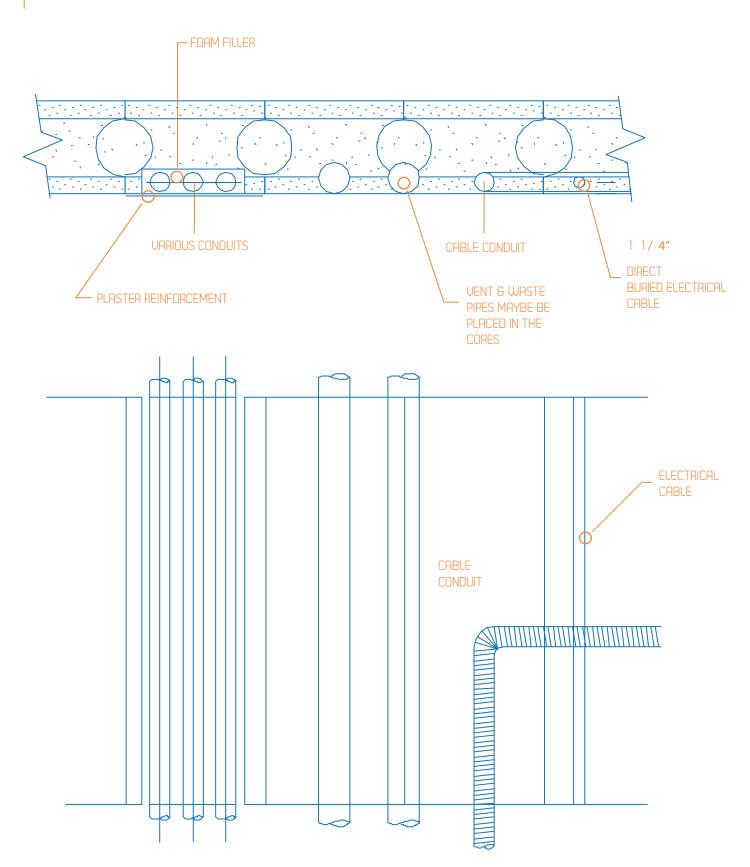








3.9 Miscellaneous Details TYPICAL CONDUIT EMBED



3.9 Miscellaneous Details TYPICAL RAIL CONNECTION

